BENDING ANALYSIS OF STATICALLY INDETERMINANT BEAMS Revision A

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Clamped-Pinned-Free Beam

Consider the uniform beam in Figure A-1. The end mass represents an applied load. Assume that the mass of the beam itself is negligible.

A free body diagram is shown in Figure A-2.



Figure A-1.



Figure A-2.

The reaction forces are given by R_a and R_b . The reaction moment is M_a .

Determine the following:

- 1. The reaction forces and reaction moment.
- 2. The displacement function and its derivatives for each span.
- 3. The moment relationship at the intermediate, pinned boundary.
- 4. The effective stiffness at the free end.

Apply Newton's law for static equilibrium.

$$+\uparrow \sum \text{forces} = 0$$
 (A-1)

$$\mathbf{R}_{\mathbf{a}} + \mathbf{R}_{\mathbf{b}} - \mathbf{m}\mathbf{g} = \mathbf{0} \tag{A-2}$$

At the left boundary,

$$+\sum$$
 moments = M_a (A-3)

$$M_a + R_b a - mg L = 0 \tag{A-4}$$

Now consider a segment of the beam, starting from the left boundary as shown in Figure A-3.



Figure A-3.

- V is the shear force.
- M is the bending moment.
- y_1 is the displacement at position x_1 .

Note that a positive displacement is downward.

Sum the moments at the right side of the segment.

$$(+\sum \text{moments} = 0$$
 (A-5)

$$\mathbf{M}_{\mathbf{a}} - \mathbf{R}_{\mathbf{a}} \mathbf{x}_{1} - \mathbf{M} = \mathbf{0} \tag{A-6}$$

$$\mathbf{M} = \mathbf{M}_{\mathbf{a}} - \mathbf{R}_{\mathbf{a}} \mathbf{x}_{\mathbf{1}} \tag{A-7}$$

The moment M and the deflection y are related by the equation

$$M = EIy_1''$$
(A-8)

where

- E is the modulus of elasticity.
- I is the area moment of inertia.

Substitute equation (A-7) into (A-8).

$$EIy_1'' = M_a - R_a x_1$$
 (A-9)

$$y_1'' = \frac{M_a - R_a x_1}{EI}$$
(A-10)

Integrate equation (A-10). A constant c_{11} is obtained.

$$y_{1}' = \frac{M_{a}x_{1} - \frac{1}{2}R_{a}x_{1}^{2}}{EI} + c_{11}$$
(A-11)

$$y_{1}' = \frac{2M_{a}x_{1} - R_{a}x_{1}^{2}}{2EI} + c_{11}$$
(A-12)

Integrate equation (A-12). A constant c_{12} is obtained.

$$y_{1} = \frac{M_{a}x_{1}^{2} - \frac{1}{3}R_{a}x_{1}^{3}}{2EI} + c_{11}x + c_{12}$$
(A-13)

$$y_1 = \frac{3M_a x_1^2 - R_a x_1^3}{6EI} + c_{11}x + c_{12}$$
(A-14)

Consider the zero displacement boundary condition at the fixed end.

$$y_1(0) = 0$$
 (A-15)

Apply the boundary condition to equation (A-14).

$$c_{12} = 0$$
 (A-16)

Consider the zero slope boundary condition at the fixed end.

$$y_1'(0) = 0$$
 (A-17)

Apply the boundary condition to equation (A-12).

$$c_{11} = 0$$
 (A-18)

Substitute equations (A-16) and (A-18) into (A-14). The displacement equation becomes

$$y_1 = \frac{3M_a x_1^2 - R_a x_1^3}{6EI}$$
(A-19)

The displacement at the pinned location is

$$y_1(a) = \frac{3M_a a^2 - R_a a^3}{6EI}$$
(A-20)

The zero displacement boundary condition at the pinned location is

$$\mathbf{y}_1(\mathbf{a}) = \mathbf{0} \tag{A-21}$$

Substitute equation (A-19) into (A-21).

$$\frac{3M_{a}a^{2} - R_{a}a^{3}}{6EI} = 0$$
 (A-22)

$$3M_a - R_a a = 0 \tag{A-23}$$

The bending moment at the fixed boundary can thus be expressed as

$$M_a = \frac{R_a a}{3} \tag{A-24}$$

Substitute equation (A-24) into (A-4).

$$\frac{R_a a}{3} + R_b a - mg L = 0 \tag{A-25}$$

$$R_a = -3R_b + mg\frac{3L}{a}$$
(A-26)

Recall equation (A-2), restated as (A-27).

$$\mathbf{R}_{\mathbf{a}} + \mathbf{R}_{\mathbf{b}} - \mathbf{m}\mathbf{g} = \mathbf{0} \tag{A-27}$$

Substitute equation (A-26) into (A-27).

$$-3R_{b} + mg\frac{3L}{a} + Rb - mg = 0$$
(A-28)

Simplify the equation by collecting terms.

$$-2R_{b} + mg\left[\frac{3L}{a} - 1\right] = 0 \tag{A-29}$$

$$R_{b} = \frac{mg}{2} \left[\frac{3L}{a} - 1 \right]$$
(A-30)

$$R_{b} = \frac{mg}{2a} [3L - a] \tag{A-31}$$

Equation (A-27) requires

$$\mathbf{R}_{a} = \mathbf{mg} - \mathbf{R}_{b} \tag{A-32}$$

Substitute equation (A-31) into (A-32).

$$R_a = mg - \frac{mg}{2a} [3L - a]$$
(A-33)

$$R_{a} = mg\left\{1 - \frac{1}{2a}\left[3L - a\right]\right\}$$
(A-34)

$$R_{a} = \frac{mg}{2a} \{ 2a - [3L - a] \}$$
 (A-35)

$$R_a = \frac{mg}{2a} \{3a - 3L\}$$
(A-36)

$$R_a = \frac{3mg}{2a} \{a - L\}$$
(A-37)

Recall that a + b = L. Thus

$$R_a = \frac{-3mgb}{2a}$$
(A-38)

Recall equation (A-24), restated as equation (A-39).

$$M_a = \frac{R_a a}{3} \tag{A-39}$$

$$M_{a} = \frac{a}{3} \frac{3mg}{2a} \{a - L\}$$
(A-40)

$$M_a = \frac{mg}{2} \{ a - L \}$$
 (A-41)

$$M_a = \frac{-mgb}{2}$$
(A-42)

Summarize the reaction forces and the reaction moment.

$$R_a = \frac{-3mgb}{2a}$$
(A-43)

$$R_b = \frac{mg}{2a} [3L - a] \tag{A-44}$$

$$M_a = \frac{-mgb}{2}$$
(A-45)

Substitute equation (A-45) and (A-43) into (A-46).

$$y_1 = \frac{-3\frac{mgb}{2}x_1^2 + \frac{3mgb}{2a}x_1^3}{6EI}$$
(A-46)

$$y_{1} = \left[\frac{mgb}{6EI}\right] \left[-\frac{3}{2}x_{1}^{2} + \frac{3}{2a}x_{1}^{3}\right]$$
(A-47)

$$y_1 = \left[\frac{mgb}{4EIa}\right] \left[-ax_1^2 + x_1^3\right]$$
(A-48)

Calculate the first derivative.

$$y_1' = \left[\frac{mgb}{4EIa}\right] \left[-2ax_1 + 3x_1^2\right]$$
 (A-49)

Evaluate the first derivative at the intermediate boundary.

$$y_1'(a) = \left[\frac{mgb}{4EIa}\right] \left[-2a^2 + 3a^2\right]$$
 (A-50)

$$y_1'(a) = \left[\frac{mg \, ab}{4 \, \text{EI}}\right] \tag{A-51}$$

Calculate the second derivative.

$$y_1'' = \left[\frac{mgb}{4EIa}\right] \left[-2a + 6x_1\right]$$
(A-52)

Evaluate the second derivative at the intermediate boundary.

$$y_1''(a) = \left[\frac{mgb}{4EIa}\right] [4a]$$

$$y_1''(a) = \left[\frac{mgb}{EI}\right]$$
(A-54)

Consider the right segment shown in Figure A-4.



Figure A-4.

Note that a positive displacement is downward.

Sum the moments at the left side of the segment.

$$+ \sum \text{moments} = 0$$
 (A-55)

$$M - mg x_2 = 0$$
 (A-56)

$$M = mg x_2 \tag{A57}$$

The moment M and the deflection y are related by the equation

$$M = EIy_2''$$
(A-58)

$$EIy_2'' = mg x_2 \tag{A-59}$$

$$y_2'' = \frac{mg}{EI} x_2$$
 (A-60)

Integrate to obtain the slope equation.

$$y_2' = \frac{mg}{2EI} x^2 + c_{21}$$
 (A-61a)

Evaluate the slope at the intermediate boundary.

$$y_2'(b) = \frac{mg}{2EI}b^2 + c_{21}$$
 (A-61b)

Integrate to obtain the displacement function.

$$y_2 = \frac{mg}{6EI} x^3 + c_{21}x + c_{22}$$
(A-62)

The displacement boundary condition at the intermediate boundary is

$$y_2(b) = 0$$
 (A-63)

Substitute equation (A-62) into (A-63).

$$\frac{\text{mg}}{6\text{EI}}b^3 + c_{21}b + c_{22} = 0 \tag{A-64}$$

$$c_{22} = -\frac{mg}{6EI}b^3 - c_{21}b \tag{A-65}$$

The slope boundary condition at the pinned boundary is

$$y'(a) = -y'(b)$$
 (A-66)

Substitute equation (A-51) and (A-61b) into (A-66).

$$\left[\frac{\text{mg ab}}{4\text{EI}}\right] = -\frac{\text{mg}}{2\text{EI}}b^2 - c_{21}$$
(A-67)

$$c_{21} = -\frac{mg}{2EI}b^2 - \left[\frac{mgab}{4EI}\right]$$
(A-68)

$$c_{21} = \frac{mg}{EI} \left\{ -\frac{1}{2} b^2 - \left[\frac{ab}{4} \right] \right\}$$
(A-69)

$$c_{21} = \frac{-mgb}{4EI} \{2b + a\}$$
(A-70)

Substitute equation (A-70) into (A-65).

$$c_{22} = -\frac{mg}{6EI}b^3 - \frac{mg}{EI}\left\{-\frac{1}{2}b^2 - \left[\frac{ab}{4}\right]\right\}b$$
(A-71)

$$c_{22} = -\frac{mg}{6EI}b^{3} - \frac{mg}{EI}\left\{-\frac{1}{2}b^{3} - \left[\frac{ab^{2}}{4}\right]\right\}$$
(A-72)

$$c_{22} = \frac{mg}{EI} \left\{ -\frac{1}{6}b^3 + \frac{1}{2}b^3 + \frac{ab^2}{4} \right\}$$
(A-73)

$$c_{22} = \frac{mg}{EI} \left\{ \frac{1}{3} b^3 + \frac{ab^2}{4} \right\}$$
(A-74)

Substitute equations (A-74) and (A-70) into (A-62).

$$y_2 = \frac{mg}{6EI} x^3 + \frac{-mgb}{4EI} \{2b + a\} x + \frac{mgb^2}{12EI} \{4b + 3a\}$$
(A-75)

$$y_2 = \frac{mg}{EI} \left\{ \frac{1}{6} x^3 + \frac{-b}{4} \{ 2b + a \} x + \frac{b^2}{12} \{ 4b + 3a \} \right\}$$
(A-76)

$$y_2 = \frac{mg}{12EI} \left\{ 2x^3 - 3b\{2b + a\}x + b^2\{4b + 3a\} \right\}$$
(A-77)

The displacement equation is now obtained. The next task is to verify the equation in terms of its boundary conditions.

Evaluate the displacement at the intermediate boundary.

$$y_{2}(b) = \frac{mg}{12EI} \left\{ 2b^{3} - 3b\{2b + a\}b + b^{2}\{4b + 3a\} \right\}$$
(A-78)

$$y_2(b) = \frac{mg}{12 \text{ EI}} \left\{ 2 b^3 - 6b^3 - 3ab^2 + 4 b^3 + 3ab^2 \right\}$$
(A-79)

$$y_2(b) = 0$$
 (A-80)

The zero displacement condition is thus verified. Recalculate the first derivative.

$$y_2' = \frac{mg}{12EI} \left\{ 6x^2 - 3b[2b+a] \right\}$$
 (A-81)

Evaluate the first derivative at the intermediate boundary.

$$y_{2}'(b) = \frac{mg}{12 \text{ EI}} \left\{ 6 b^{2} - 3b[2b + a] \right\}$$
 (A-82)

$$y_{2}'(b) = \frac{mg}{12EI} \left\{ 6b^2 - 6b^2 - 3ab \right\}$$
 (A-83)

$$y_2'(b) = \frac{mgab}{-4EI}$$
(A-84)

The slope equation (A-66) is thus verified.

Recalculate the second derivative.

$$y_2" = \frac{mg}{12EI} \{12x\}$$
 (A-85)

$$y_2" = \frac{mgx}{EI}$$
(A-86)

Evaluate the second derivative at the intermediate boundary.

$$y_2"(b) = \frac{mgb}{EI}$$
(A-87)

Recall equation (A-54).

$$y_1''(a) = \left[\frac{mgb}{EI}\right]$$
(A-88)

Compare equations (A-87) and (A-88).

$$y_1''(a) = y_2''(b)$$
 (A-89)

Equation (A-89) is the bending moment relationship at the intermediate boundary.

Now consider the special case where

$$a = b = \frac{L}{2} \tag{A-90}$$

Let

$$Y(x) = \begin{cases} -y_1(x_1) & \text{for } 0 \le x \le a \\ -y_2(x_2) & \text{for } a < x \le L \end{cases}$$
(A-91)

The negative sign changes the positive displacement to the upward direction.

The resulting displacement function is plotted in Figure A-5.

The displacement at the free end is

$$y_2(0) = \frac{mg}{12 \text{ EI}} \left\{ b^2 \{ 4 b + 3a \} \right\}$$
 (A-92)

Hooke's law for a force F and stiffness k is

$$\mathbf{F} = \mathbf{k} \mathbf{y} \tag{A-93}$$

$$k = \frac{F}{y}$$
(A-94)

The applied force at the free end is

$$\mathbf{F} = \mathbf{mg} \tag{A-95}$$

The effective stiffness at the free end is thus

$$k = \frac{mg}{\frac{mg}{12EI} \left\{ b^{2} \left[4 \ b + 3a \right] \right\}}$$
(A-96)
$$k = \frac{12EI}{\left\{ b^{2} \left[4 \ b + 3a \right] \right\}}$$
(A-97)

CLAMPED-PINNED-FREE BEAM WITH END MASS DISPLACEMENT SHAPE



Figure A-5.